

Lecture 6

Maxwell's rule and Grübler's formula

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DoF formula extended to compliant mechanisms

Midha, Murphy, and Howell (1995)

Ananthasuresh and Howell (1996)

3D

$$DoF = 6(n_{seg} - 1) - \sum_{j=1}^5 (6 - j) n_{Kj} - \sum_{j=1}^5 (6 - j) n_{Cj} - 6n_{fix} + \sum_{j=1}^6 j n_{scj}$$

2D

$$DoF = 3(n_{seg} - 1) - \sum_{j=1}^2 (3 - j) n_{Kj} - \sum_{j=1}^2 (3 - j) n_{Cj} - 3n_{fix} + \sum_{j=1}^3 j n_{scj}$$

n_{seg} = number of **segments** (rigid or compliant)

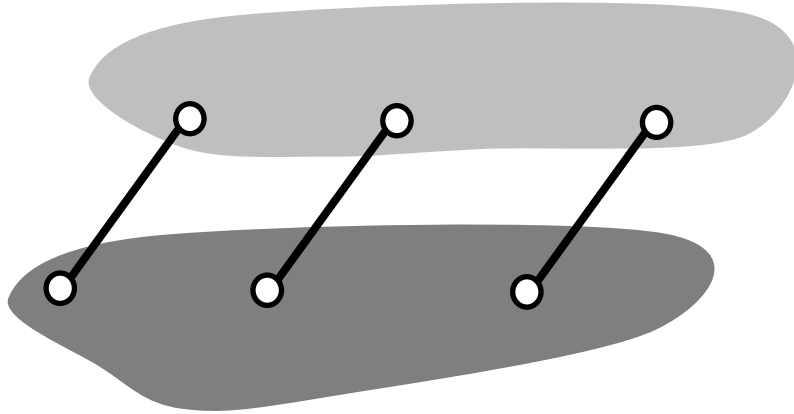
n_{Kj} = number of kinematic pairs allowing j relative dof

n_{Cj} = number of **elastic pairs** allowing j relative dof

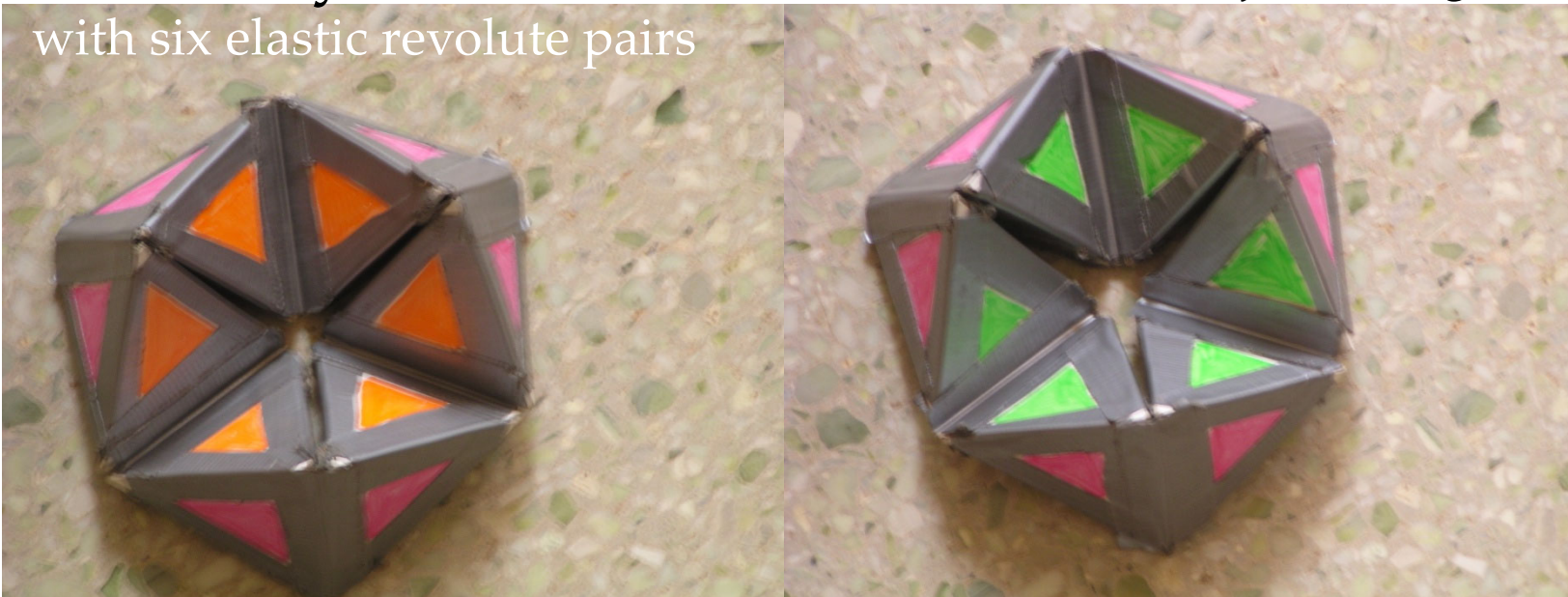
n_{fix} = number of **fixed connections**

n_{scj} = number of segments with **segment compliance** of j

An example that does not work

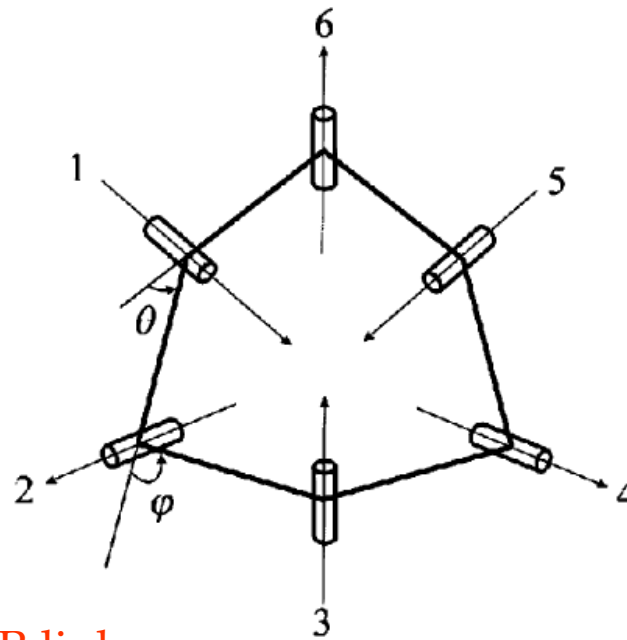
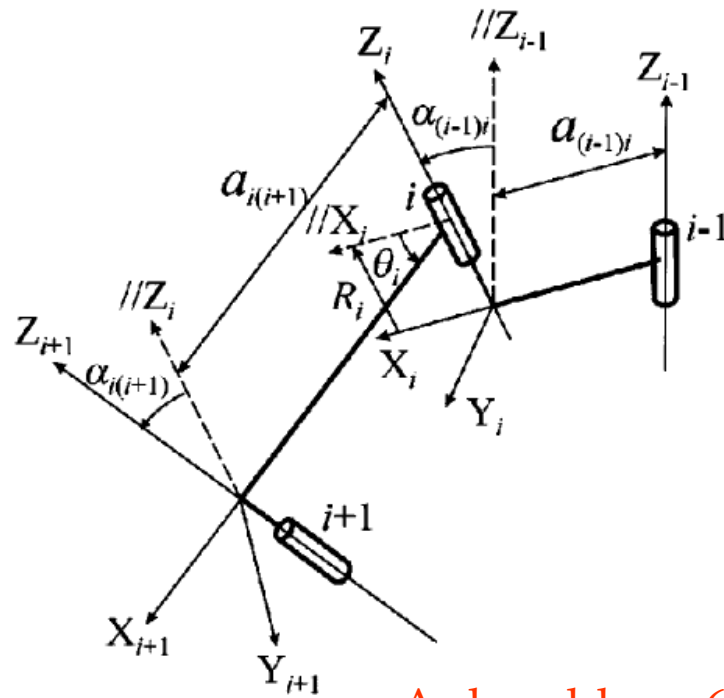


Kaleidocycle mechanism: six-tetrahedrons joined together with six elastic revolute pairs



Bricard linkage: the formula fails!

$$\begin{aligned}
 DoF &= 6(n-1) - 5f_1 - 4f_2 - 3f_3 - 2f_4 - f_5 \\
 &= 6(6-1) - (5 \times \mathbf{6}) - (4 \times \mathbf{0}) - (3 \times \mathbf{0}) - (2 \times \mathbf{0}) - (1 \times \mathbf{0}) \\
 &= 30 - 30 = \mathbf{0}
 \end{aligned}$$



A closed-loop 6R linkage

Maxwell's rule

—structures perspective

2D

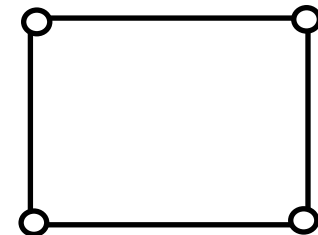
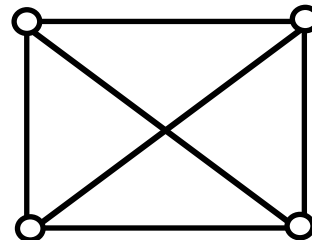
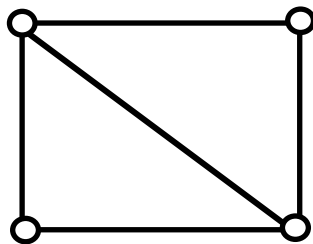
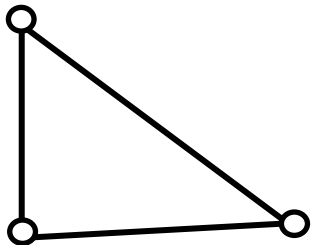
$$2v - 3 - b = 0$$

v = number of vertices

3D

$$3v - 6 - b = 0$$

b = number of bars



Maxwell's rule

— structures perspective

2D

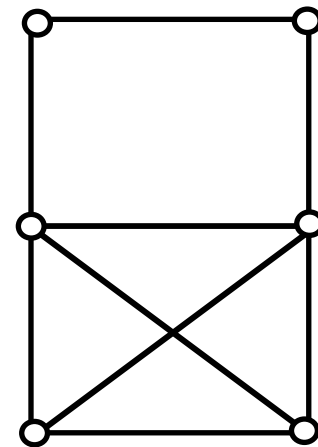
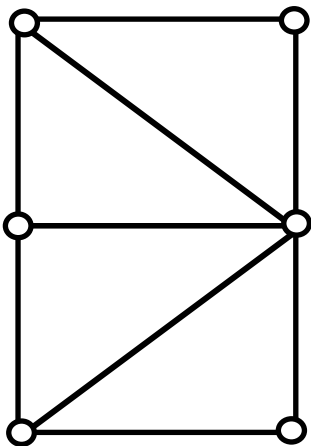
$$2v - 3 - b = 0$$

v = number of vertices

3D

$$3v - 6 - b = 0$$

b = number of bars



Maxwell's rule

—structures perspective

2D

$$2v - 3 - b = DoF - SoSS$$

Modified by Calladine

3D

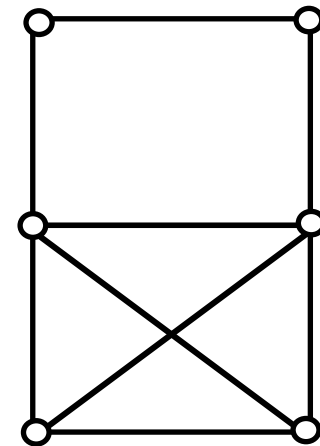
$$3v - 6 - b = DoF - SoSS$$

v = number of vertices

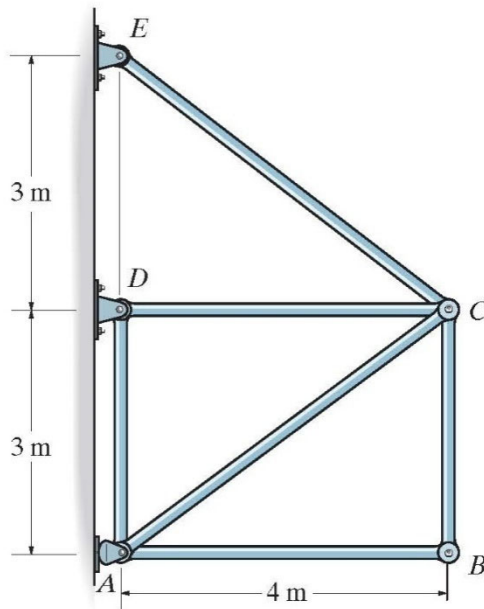
b = number of bars

DoF = number of degrees of freedom

SoSS = number of states of self-stress

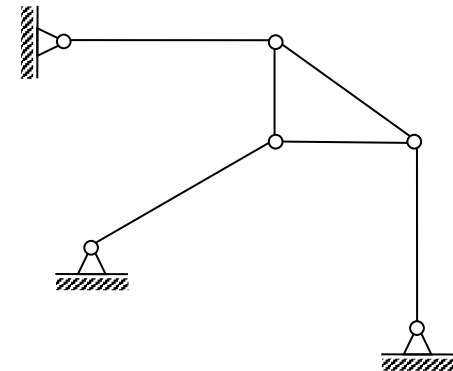
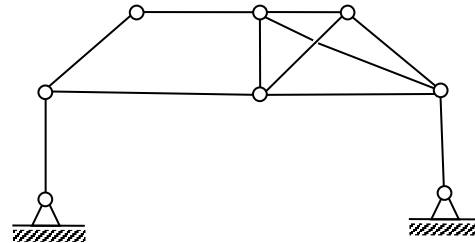


Maxwell's rule applied to trusses



$$2v - 3 - b = DoF - SoSS$$

$$16 - 3 - 12 = 1 = 2 - 1$$



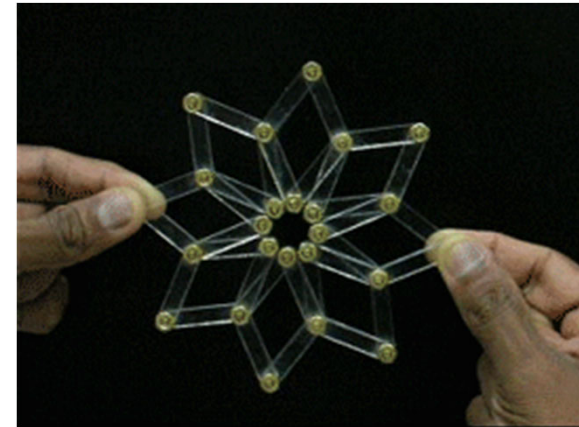
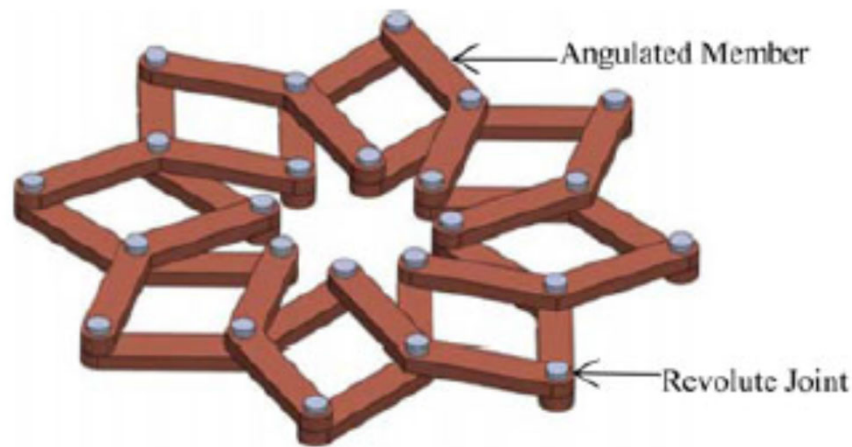
$$2v - 3 - b = DoF - SoSS$$

$$10 - 3 - 7 = 0$$

$$2v - 3 - b = DoF - SoSS$$

$$12 - 3 - 9 = 0$$

Maxwell's rule can be applied to trusses and linkages alike...



$$2v - 3 - b = DoF - SoSS$$

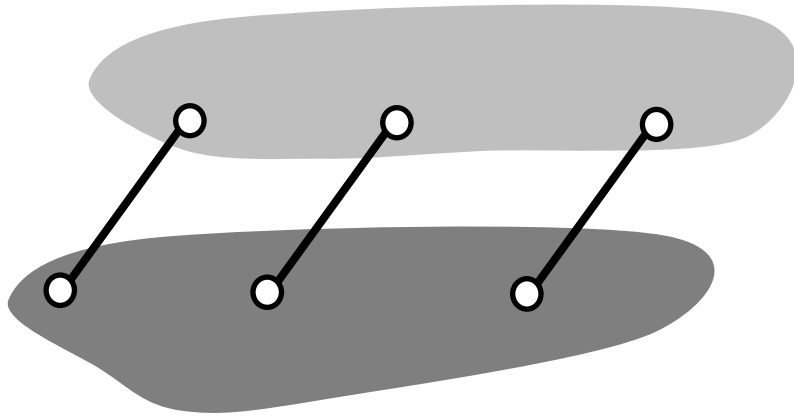
$$48 - 3 - 48 = -3$$

$$DoF = 3(n - 1) - 2n_{K1}$$

$$DoF = 3(16 - 1) - 2 \times 24 = -3$$

Fowler and **Guest** have developed nice theory for counting **symmetry groups** and resolve the conflicts arising due to special geometric conditions.

An example that does not work



Let us understand the Maxwell's rule

2D

$$2v - 3 - b = 0$$

Equilibrium matrix

$$\begin{array}{ccccc} \text{Equilibrium} & & & & \text{Forces at} \\ \text{matrix} & & & & \text{vertices} \\ & \text{Bar forces} & & & \\ \mathbf{H}_{2v \times b} \mathbf{p}_{b \times 1} & = & \mathbf{f}_{2v \times 1} \end{array}$$

Compatibility matrix

Compliance matrix	Disp. at vertices	Bar elongations
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$$\mathbf{C}_{b \times 2v} \mathbf{u}_{2v \times 1} = \mathbf{e}_{b \times 1}$$

Compatibility and Equilibrium matrices

$$\begin{array}{ccccc} \text{Compliance} & & \text{Disp. at} & & \text{Bar} \\ \text{matrix} & & \text{vertices} & & \text{elongations} \\ \mathbf{C}_{b \times 2v} & \mathbf{u}_{2v \times 1} & = & \mathbf{e}_{b \times 1} \end{array}$$

$$\begin{array}{ccccc} \text{Equilibrium} & & & & \text{Forces at} \\ \text{matrix} & & \text{Bar forces} & & \text{vertices} \\ \mathbf{H}_{2v \times b} & \mathbf{p}_{b \times 1} & = & \mathbf{f}_{2v \times 1} \end{array}$$

$$\mathbf{p}^T \delta \mathbf{e} = \mathbf{f}^T \delta \mathbf{u}$$

$$\Rightarrow \mathbf{p}^T \mathbf{C} \delta \mathbf{u} = \mathbf{p}^T \mathbf{H}^T \delta \mathbf{u}$$

$$\Rightarrow \mathbf{C} = \mathbf{H}^T$$

Compatibility and Equilibrium matrices

$$\begin{array}{ccccc} \text{Compliance} & & \text{Disp. at} & & \text{Bar} \\ \text{matrix} & & \text{vertices} & & \text{elongations} \\ \mathbf{C}_{b \times 2v} & \mathbf{u}_{2v \times 1} & = & \mathbf{e}_{b \times 1} \end{array}$$

$$\begin{array}{ccccc} \text{Equilibrium} & & & & \text{Forces at} \\ \text{matrix} & & \text{Bar forces} & & \text{vertices} \\ \mathbf{H}_{2v \times b} & \mathbf{p}_{b \times 1} & = & \mathbf{f}_{2v \times 1} \end{array}$$

$$\mathbf{p}^T \delta \mathbf{e} = \mathbf{f}^T \delta \mathbf{u}$$

$$\Rightarrow \mathbf{p}^T \mathbf{C} \delta \mathbf{u} = \mathbf{p}^T \mathbf{H}^T \delta \mathbf{u}$$

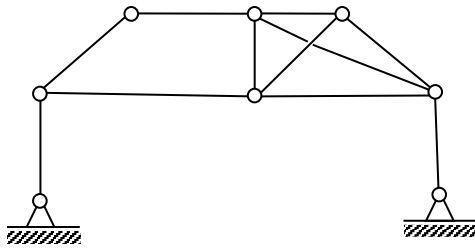
$$\Rightarrow \mathbf{C} = \mathbf{H}^T$$

Rank-deficiency of \mathbf{C} indicates DoF
Rank-deficiency of \mathbf{H} indicates SoSS.

Null-space of \mathbf{C} indicates
instantaneous rigid-body modes.

Null-space of \mathbf{H} indicates self-stress
modes.

DoF and SoSS



$$2v - 3 - b = DoF - SoSS$$

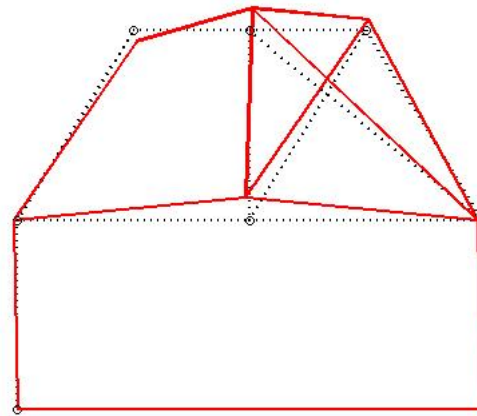
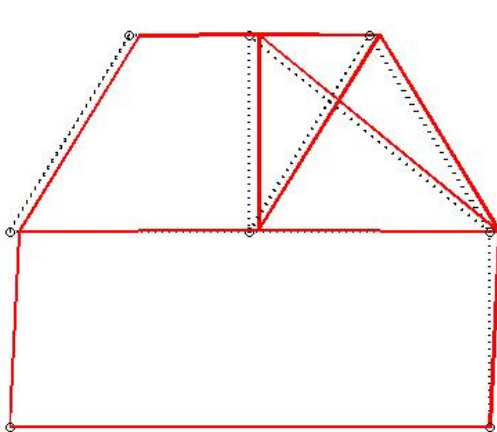
$$16 - 3 - 12 = 1 = 2 - 1$$

$$\mathbf{C}_{12 \times 16} \mathbf{u}_{16 \times 1} = \mathbf{e}_{12 \times 1}$$

Rank deficiency = 2 (not counting rigid-body modes)
 \rightarrow 2 DoF

$$\mathbf{H}_{16 \times 12} \mathbf{p}_{12 \times 1} = \mathbf{f}_{16 \times 1}$$

Rank deficiency = 1
 \rightarrow 1 SoSS



Null-space “modes” of \mathbf{C} .

Can also use stiffness matrix. (finite element framework)

Compliance matrix Disp. at vertices Bar elongations

$$\mathbf{C}_{b \times 2v} \mathbf{u}_{2v \times 1} = \mathbf{e}_{b \times 1}$$

Equilibrium matrix Bar forces Forces at vertices

$$\mathbf{H}_{2v \times b} \mathbf{p}_{b \times 1} = \mathbf{f}_{2v \times 1}$$

$$\mathbf{p}_{b \times 1} = \mathbf{D}_{b \times b} \mathbf{e}_{b \times 1}$$

$$\mathbf{K}_{2v \times 2v} \mathbf{u}_{2v \times 1} = \mathbf{f}_{2v \times 1}$$

$$\Rightarrow \mathbf{p}_{b \times 1} = \mathbf{D}_{b \times b} \mathbf{C}_{b \times 2v} \mathbf{u}_{2v \times 1}$$

$$\Rightarrow \mathbf{H}_{2v \times b} \mathbf{D}_{b \times b} \mathbf{C}_{b \times 2v} \mathbf{u}_{2v \times 1} = \mathbf{H}_{2v \times b} \mathbf{p}_{b \times 1}$$

$$\Rightarrow \mathbf{H}_{2v \times b} \mathbf{D}_{b \times b} \mathbf{C}_{b \times 2v} \mathbf{u}_{2v \times 1} = \mathbf{f}_{2v \times 1}$$

Rank deficiency of the stiffness matrix

$$\mathbf{K}_{2v \times 2v} \mathbf{u}_{2v \times 1} = \mathbf{f}_{2v \times 1}$$

Main Points

- Maxwell's rule and Grübler's formula are equivalent for trusses/linkages.
- Compatibility and equilibrium matrices give correct but only instantaneous DoF and SoSS.
- Stiffness matrix can also be used for finding instantaneous (infinitesimal) DoF.

Further reading



International Journal of Solids and Structures

Volume 14, Issue 2, 1978, Pages 161-172



Buckminster Fuller's “Tensegrity” structures and Clerk Maxwell's rules for the construction of stiff frames

C.R. Calladine

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