

# Lecture 8b

# The dual problem of size optimization of statically determinate trusses

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ME260 Indian Institute of Science

**Structural Optimization: Size, Shape, and Topology**

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# Outline of the lecture

Maxwell's rule for statically determinate trusses

Dual problem for statically determinate trusses

What we will learn:

Maxwell's rule with Calladine's modification

Posing the dual problem of size optimization of statically determinate trusses

# Statically determinate trusses

*Statically determinate trusses are those in which internal forces can be computed from equations of statics without having to solve for displacements.*

*→ Internal forces do not depend on areas of cross section of the truss members.*

*Statically determinate trusses satisfy the Maxwell's rule.*

## Maxwell's rule

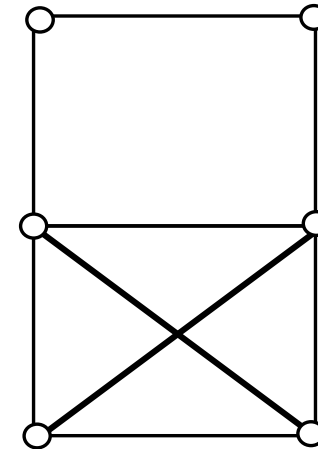
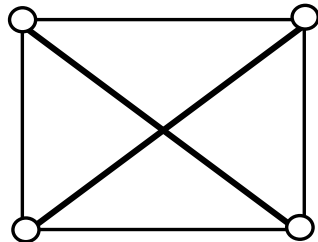
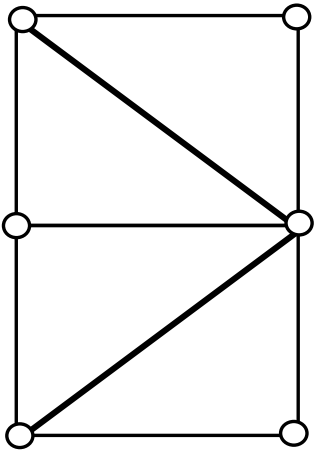
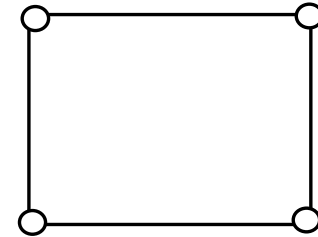
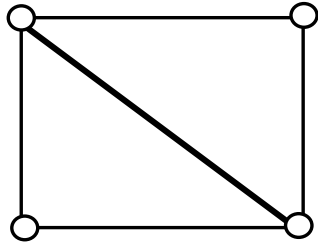
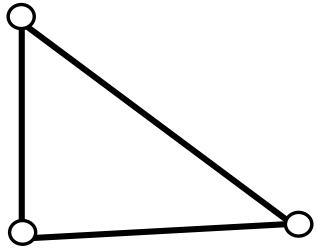
$$2D \quad 2v - 3 - b = 0$$

$v$  = number of **vertices**

$$3D \quad 3v - 6 - b = 0$$

$b$  = number of **bars**

# Try Maxwell's rule on these



# Maxwell's rule modified by Calladine

2D

$$2v - 3 - b = DoF - SoSS$$

3D

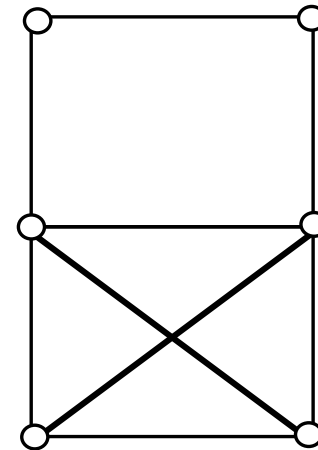
$$3v - 6 - b = DoF - SoSS$$

$v$  = number of vertices

$b$  = number of bars

DoF = number of degrees of freedom

SoSS = number of states of self-stress



# How do you interpret Maxwell's rule?

$$2D \quad 2v - 3 - b = 0$$

Equilibrium  
matrix

$$\mathbf{H}_{2v \times b} \mathbf{p}_{b \times 1} = \mathbf{f}_{2v \times 1}$$

Bar forces

Forces at  
vertices

Compatibility  
matrix

$$\mathbf{C}_{b \times 2v} \mathbf{u}_{2v \times 1} = \mathbf{e}_{b \times 1}$$

Disp. at  
vertices

Bar  
elongations

$$\mathbf{p}^T \delta \mathbf{e} = \mathbf{f}^T \delta \mathbf{u}$$

$$\Rightarrow \mathbf{p}^T \mathbf{C} \delta \mathbf{u} = \mathbf{p}^T \mathbf{H}^T \delta \mathbf{u}$$

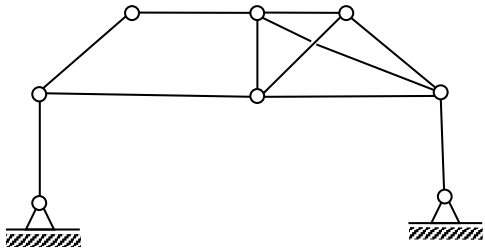
$$\Rightarrow \mathbf{C} = \mathbf{H}^T$$

Rank-deficiency of  $\mathbf{C}$  indicates DoF  
Rank-deficiency of  $\mathbf{H}$  indicates SoSS.

Null-space of  $\mathbf{C}$  indicates instantaneous  
rigid-body modes.

Null-space of  $\mathbf{H}$  indicates self-stress modes.

# DoF and SoSS



$$2v - 3 - b = DoF - SoSS$$

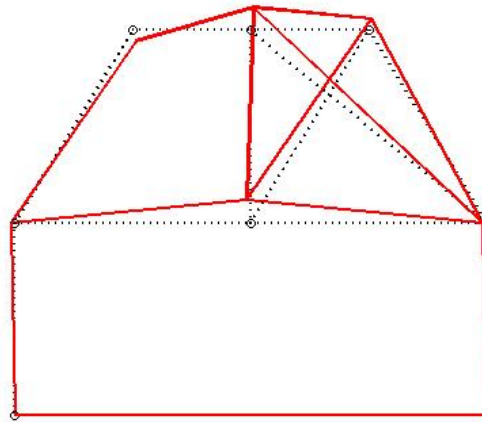
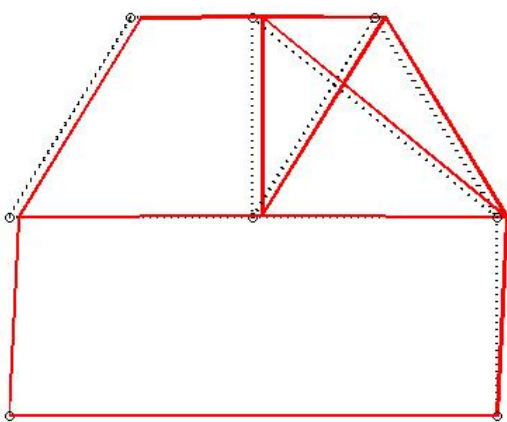
$$16 - 3 - 12 = 1 = 2 - 1$$

$$\mathbf{C}_{12 \times 16} \mathbf{u}_{16 \times 1} = \mathbf{e}_{12 \times 1}$$

Rank deficiency = 2 (not counting rigid-body modes)  
 → 2 DoF

$$\mathbf{H}_{16 \times 12} \mathbf{p}_{12 \times 1} = \mathbf{f}_{16 \times 1}$$

Rank deficiency = 1  
 → 1 SoSS



Null-space “modes” of  $\mathbf{C}$ .

# We can also use the stiffness matrix (finite element framework)

Compatibility matrix	Disp. at vertices	Bar elongations	Equilibrium matrix	Bar forces	Forces at vertices
$\mathbf{C}_{b \times 2v}$	$\mathbf{u}_{2v \times 1}$	$= \mathbf{e}_{b \times 1}$	$\mathbf{H}_{2v \times b}$	$\mathbf{p}_{b \times 1}$	$= \mathbf{f}_{2v \times 1}$

$$\mathbf{p}_{b \times 1} = \mathbf{D}_{b \times b} \mathbf{e}_{b \times 1}$$

$$\mathbf{K}_{2v \times 2v} \mathbf{u}_{2v \times 1} = \mathbf{f}_{2v \times 1}$$

$$\Rightarrow \mathbf{p}_{b \times 1} = \mathbf{D}_{b \times b} \mathbf{C}_{b \times 2v} \mathbf{u}_{2v \times 1}$$

$$\Rightarrow \mathbf{H}_{2v \times b} \mathbf{D}_{b \times b} \mathbf{C}_{b \times 2v} \mathbf{u}_{2v \times 1} = \mathbf{H}_{2v \times b} \mathbf{p}_{b \times 1}$$

$$\Rightarrow \mathbf{H}_{2v \times b} \mathbf{D}_{b \times b} \mathbf{C}_{b \times 2v} \mathbf{u}_{2v \times 1} = \mathbf{f}_{2v \times 1}$$



# Rank deficiency of the stiffness matrix

$$\mathbf{K}_{2\nu \times 2\nu} \mathbf{u}_{2\nu \times 1} = \mathbf{f}_{2\nu \times 1}$$

## Summary

Compatibility and equilibrium matrices give correct but only instantaneous DoF and SoSS.

Stiffness matrix can also be used for finding instantaneous (infinitesimal) DoF.

# Size optimization of statically determinate trusses

$$\text{Min}_{\mathbf{a}} MC = \sum_{i=1}^n P_i u_i = \mathbf{p}^T \mathbf{u}$$

Subject to

$$\lambda: \quad \mathbf{K}\mathbf{u} - \mathbf{p} = 0$$

$$\mu: \quad \rho \mathbf{a}^T \mathbf{1} - W^* \leq 0$$

$$\text{Data: } \rho, W^*, l_{i=1,2,\dots,N}, N, P_{i=1,2,\dots,N}, E$$

$$\text{Min}_{\mathbf{a}} MC = \mathbf{p}^T \mathbf{u} = \sum_{i=1}^n P_i u_i$$

Subject to

$$\mu: \quad \rho \mathbf{a}^T \mathbf{1} - W^* \leq 0$$

$$\text{Data: } \rho, W^*, l_{i=1,2,\dots,N}, N, P_{i=1,2,\dots,N}, E$$

$$u_i = (\delta \mathbf{u}^T \mathbf{K}) \mathbf{u}^*$$

# Contd.

$$\text{Min}_{\mathbf{a}} MC = \mathbf{p}^T \mathbf{u} = \sum_{i=1}^n P_i u_i$$

Subject to

$$\mu: \quad \rho \mathbf{a}^T \mathbf{l} - W^* \leq 0$$

Data:  $\rho, W^*, l_{i=1,2,\dots,N}, N, P_{i=1,2,\dots,N}, E$

$$L = \sum_{i=1}^n P_i \left( \sum_{j=1}^N t_j^{(i)} \frac{T_j l_j}{A_j E} \right) + \mu \sum_{j=1}^N \rho A_j l_j$$

$$\frac{\partial L}{\partial A_k} = - \sum_{i=1}^n P_i \frac{t_j^{(i)} T_k l_k}{A_k^2 E} + \rho \mu l_k = 0$$

$$u_i = (\delta \mathbf{u}^T \mathbf{K}) \mathbf{u}^* = \sum_{j=1}^N t_j^{(i)} \frac{T_j l_j}{A_j E}$$

$T_j =$  Internal force in  $j^{\text{th}}$  truss member due to applied real loads.

$t_j^{(i)} =$  Internal force in  $j^{\text{th}}$  truss member due to unit virtual load applied on  $i^{\text{th}}$  DoF.

This enables us to obtain an expression for each area of cross section in terms of data and  $\mu$

# Dual problem

$$\text{Max}_{\mu} L(\mu)$$

Now, it is a one variable unconstrained maximization problem except that  $\mu$  should be non-negative.

Note that we can do this for a statically determinate truss, however large it may be.

# We have two methods now.

General algorithms with outer and inner loops to find cross section areas of any kind of truss.

We have the Matlab code for this wherein we begin with an exhaustive “ground structure” with all truss elements defined between every pair of points in a grid of vertices.

We should check whether the resulting optimal truss is statically determinate or no.

A specific dual formulation that reduces the size optimization of statically determinate trusses to a one-variable problem.

Here, we need to first check if the truss is statically determinate or not using the Maxwell’s rule and then with the rank-deficiency of the equilibrium matrix.

# The end note

- Truss optimization
- Statically determinate trusses
  - Maxwell's rule for static determinacy and Calladine's modification
  - Degrees of freedom (DoF) and states of self stress (SoSS)
  - Force equilibrium and displacement compatibility matrices
  - Rank deficiency of the force equilibrium gives the number of SoSS and that of displacement compatibility gives the number of DoF.**
  - Dual problem for statically determinate trusses.

Thanks